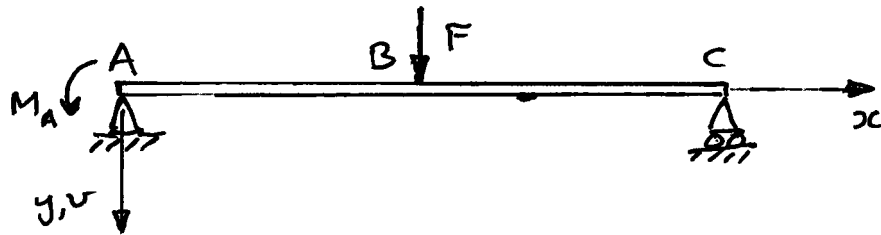
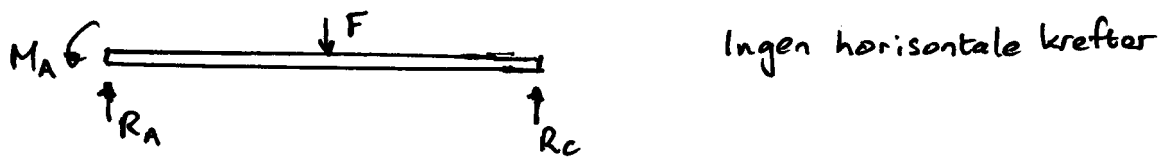


Oppgave 1



(a) Fritt legemediagram for hele bjelken:



$$\sum M_A = 0 \Rightarrow M_A + R_C L - F L/2 = 0$$

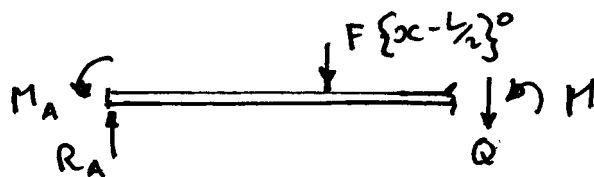
$$\underline{\underline{R_C = \frac{F}{2} - \frac{M_A}{L}}}$$

$$\sum F_y = 0 \Rightarrow R_A + R_C - F = 0$$

$$R_A = F - \left(\frac{F}{2} - \frac{M_A}{L}\right) = \underline{\underline{\frac{F}{2} + \frac{M_A}{L}}}$$

(b)  $N=0$  for alle  $x$ -verdier.

For å finne  $M, Q$  tar vi et utsnitt og bruker Macaulay-notasjonen



$$\sum F_y = 0 \Rightarrow R_A - Q - F \{x - L/2\}^0 = 0$$

$$Q = \frac{F}{2} + \frac{M_A}{L} - F \{x - L/2\}^0$$

$$\sum M_{\text{snitt}} \Rightarrow M_A - R_A x + F \{x - L/2\} + M = 0$$

$$M = -M_A + R_A x - F \{x - L/2\}$$

$$= -M_A + \left(\frac{F}{2} + \frac{M_A}{L}\right)x - F \{x - L/2\}$$

$$= M_A \left(\frac{x}{L} - 1\right) + F \left(\frac{x}{2} - \{x - L/2\}\right)$$

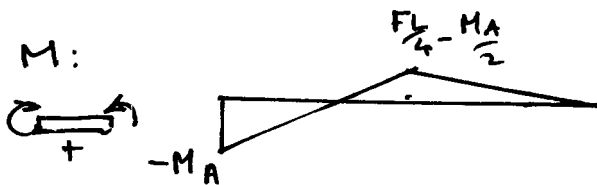
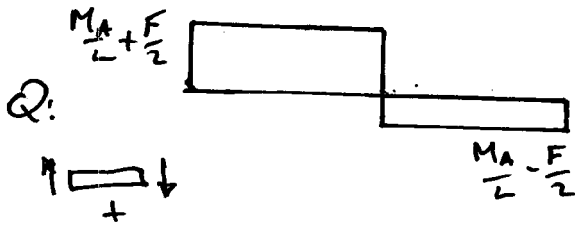
Ved  $x \leq L/2$  er  $Q = \frac{F}{2} + \frac{M_A}{L}$ , ved  $x > L/2$  er  $Q = \frac{M_A}{L} - \frac{F}{2}$

(2)

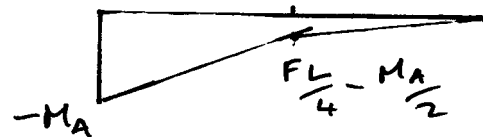
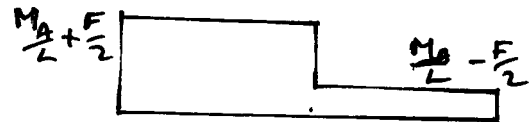
Ved  $x=L/2$  er  $M = \frac{FL}{4} - \frac{M_A}{2}$

Formen til Q- og M-diagrammene er afhængig af om  $\frac{F}{2} < \frac{M_A}{L}$  eller  $\frac{F}{2} > \frac{M_A}{L}$ . Vi viser begge varianter:

Med  $\frac{F}{2} < \frac{M_A}{L}$ :



Med  $\frac{F}{2} > \frac{M_A}{L}$ :



(c)

$$M = -EI u''$$

$$-EI u'' = M_A \left( \frac{x}{L} - 1 \right) + F \left( \frac{x}{2} - \left\{ x - \frac{L}{2} \right\} \right)$$

Integreres 2 gange:

$$-EI u' = M_A \left( \frac{1}{2} \frac{x^2}{L} - x \right) + F \left( \frac{1}{4} x^2 - \frac{1}{2} \left\{ x - \frac{L}{2} \right\}^2 \right) + C_1$$

$$-EI u = M_A \left( \frac{1}{6} \frac{x^3}{L} - \frac{1}{2} x^2 \right) + F \left( \frac{1}{12} x^3 - \frac{1}{6} \left\{ x - \frac{L}{2} \right\}^3 \right) + C_1 x + C_2$$

Randbetingelser:

$$u(0) = 0 \Rightarrow 0 = C_2$$

$$u(L) = 0 \Rightarrow 0 = M_A \left( -\frac{1}{3} L^2 \right) + F \left( \frac{1}{12} - \frac{1}{6} \cdot \frac{1}{8} \right) L^3 + C_1 L$$

$$\Rightarrow C_1 = \frac{1}{3} M_A L - \frac{1}{16} F L^2$$

Dermed blir

$$u = \frac{1}{EI} \left[ F \left( \frac{1}{16} L^2 x - \frac{1}{6} \left\{ x - \frac{L}{2} \right\}^3 - \frac{1}{12} x^3 + M_A \left( \frac{1}{6} \frac{x^3}{L} - \frac{1}{2} x^2 - \frac{1}{3} L x \right) \right) \right]$$

$$= \frac{1}{EI} \left[ \frac{F}{48} (3L^2 x - 4x^3 + 8 \{x - L/2\}^3) - \frac{M_A}{6L} (x^3 - 3x^2 L - 2xL^2) \right]$$

③

$$(d) M_A = 0 \Rightarrow M = F \left( \frac{x}{2} - \left\{ x - \frac{L}{2} \right\} \right)$$

$\sigma_x$  har maks.verdi når  $|M|$  har maks.verdi. Det er klart dette skjer ved  $x = L/2$ .

$$M_{\text{maks}} = FL/4$$

$$\sigma = \frac{M_y}{I}$$

$$\sigma_{\text{maks}} = M_{\text{maks}} \frac{y_{\text{maks}}}{I}$$

$$I = \frac{1}{12} bh^3, \quad y_{\text{maks}} = \frac{h}{2}, \quad M_{\text{maks}} = FL/4$$

$$\sigma_{\text{maks}} = \frac{FL}{4} \cdot \frac{h}{2} \cdot \frac{12}{bh^3} = \underline{\underline{\frac{3}{2} \frac{FL}{bh^2}}}$$

(e) Nå bruker vi resultatene fra del (c) og setter  $v'(0) = 0$ :

$$0 = M_A \cdot 0 + F \{0\} + C_1$$

altså  $C_1 = 0$

Men  $C_1 = \frac{1}{3} M_A L - \frac{1}{16} FL^2$  fra før, så

$$\frac{1}{3} M_A L = \frac{1}{16} FL^2$$

$$\underline{\underline{M_A = \frac{3}{16} FL}}$$

Da får vi også

$$R_A = \frac{F}{2} + \frac{M_A}{L} = F \left( \frac{1}{2} + \frac{3}{16} \right) = \underline{\underline{\frac{11}{16} F}}$$

og  $\underline{\underline{R_C = \frac{5}{16} F}}$

Oppgave 2

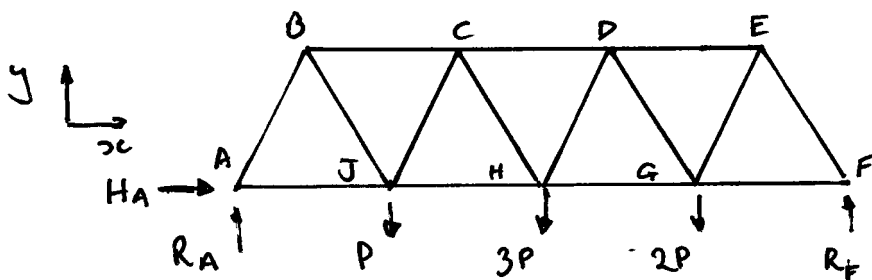
(a) Figur 3(a) har 12 staver med ukjente aksialkrefter, samt 3 ukjente reaksjonskrefter, dvs. 15 ukjente.

Det er 8 knutepunkter  $\Rightarrow$  16 uavhengige likevektsligninger.  
 Dette fagverket er statisk underbestemt (mekanisme).

Det kreves en ekstra stav diagonalt i midten for å gjøre dette fagverket statisk bestemt.

Figur 3(b) er som 3(a), men med 14 staver med ukjente aksialkrefter. Nå har vi 17 ukjente og kun 16 uavhengige likevektsligninger  $\Rightarrow$  statisk ubestemt.

(b)



$$\sum F_x = 0 \quad \Rightarrow \quad H_A = 0$$

$$\sum F_y = 0 \quad \Rightarrow \quad R_A + R_F = P + 3P + 2P = 6P$$

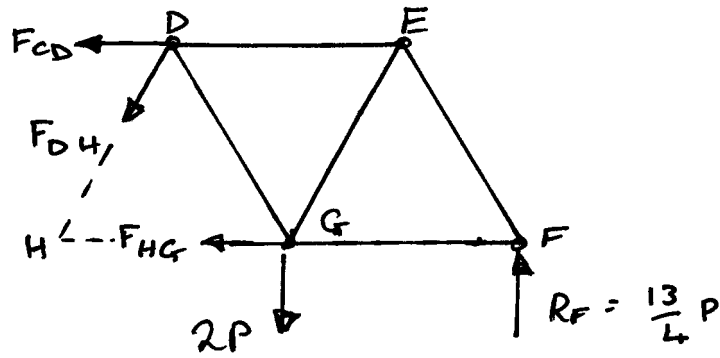
$$\sum M_A = 0 \quad \Rightarrow \quad 4a R_F = a \cdot P + 2a \cdot 3P + 3a \cdot 2P$$

$$R_F = \frac{13}{4} P$$

$$R_A = 6P - R_F = \frac{11}{4} P$$

Vi tar et snitt gjennom CD, DH og HG og tegner et fritt legemediagram for den delen av fagverket til høyre for snittet:

(5)



$$\sum F_y = 0 \Rightarrow F_{DH} \cos 30^\circ + 2P - R_F = 0$$

$$F_{DH} = \frac{2}{\sqrt{3}} \left( \frac{13}{4} P - 2P \right) = \frac{2}{\sqrt{3}} \cdot \frac{5}{4} P$$

$$= \frac{5}{2\sqrt{3}} P = \underline{\underline{1.443 P}}$$

$$\sum M_H = 0 \Rightarrow F_{CD} a \cos 30^\circ - 2Pa + R_F 2a = 0$$

$$F_{CD} = \frac{2}{\sqrt{3}} \left( 2P - \frac{13}{4} P \cdot 2 \right) = -\frac{2}{\sqrt{3}} \cdot \frac{9}{2} P$$

$$= -3\sqrt{3} P = \underline{\underline{-5.196 P}}$$

$$\sum M_D = 0 \Rightarrow -F_{HG} a \cos 30^\circ - 2P \cdot \frac{a}{2} + R_F \cdot \frac{3a}{2} = 0$$

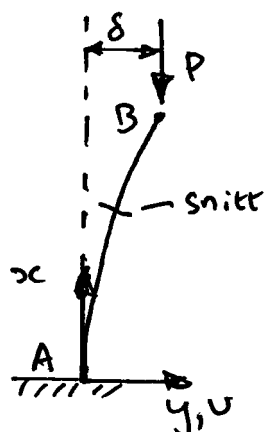
$$F_{HG} = \frac{2}{\sqrt{3}} \left( \frac{3}{2} \cdot \frac{13}{4} P - P \right) = \frac{2}{\sqrt{3}} \cdot \frac{31}{8} P$$

$$= \frac{31}{4\sqrt{3}} P = \underline{\underline{4.474 P}}$$

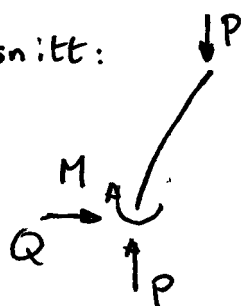
Oppgave 3

Vi kan sette opp aksekorset med origo i punkt A eller B (som beveger seg med søylen). Her viser vi løsningen med origo i punkt A.

(a) Deformert søyle:



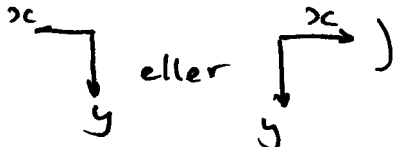
Utsnitt:



$$\sum F_y = 0 \Rightarrow Q = 0$$

$$\sum M_{\text{snitt}} = 0$$

$$\Rightarrow M + P(\delta - u) = 0$$

(NB: Aksekorset tegnes ofte slikt: )

Fra ligningen for bøyning på formelarket får vi

$$\begin{aligned} -EIv'' &= M \\ &= -P(\delta - u) \end{aligned}$$

$$v'' + \frac{P}{EI}v = \frac{P\delta}{EI}$$

Vi setter  $k^2 = P/EI$

Da blir løsningen  $v = C_1 \sin kx + C_2 \cos kx + \delta$

Randbetingelser:

$$v(0) = 0 \Rightarrow C_2 + \delta = 0 \Rightarrow C_2 = -\delta$$

$$v'(0) = 0 \Rightarrow C_1 k \cos(0) - C_2 k \sin(0) = 0 \Rightarrow C_1 = 0$$

$$v(L) = \delta \Rightarrow C_1 \sin(kL) + C_2 \cos(kL) + \delta = \delta$$

$$\Rightarrow C_2 \cos(kL) = 0$$

Dette betyr at enten er  $C_2 = 0$  eller  $\cos(kL) = 0$

$C_2 = 0$  betyr ingen deformasjon, uansett verdien av lasten  $P$ .  
 $\cos(kL) = 0$  betyr at  $kL = \frac{\pi}{2}, \frac{3\pi}{2}, \dots$  mens  $C_2$  er ubestemt. I dette tilfellet er

$$P = k^2 EI = \frac{n^2 \pi^2 EI}{4L^2}, \quad n = 1, 3, 5, \dots$$

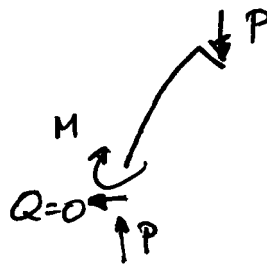
Den laveste verdien gir kritisk last

$$P_{cr} = \frac{\pi^2 EI}{4L^2}$$

(b) Med eksentrisk last har vi følgende:



utsnitt:



$\sum M = 0$  gir nå  
 $M + P(e + \delta - u) = 0$

$$-EI u'' = M = -P(e + \delta - u)$$

$$u'' + \frac{P}{EI} u = \frac{P}{EI} (\delta + e)$$

Løsningen blir nå  $u = C_1 \sin kx + C_2 \cos kx + \delta + e$   
 med  $k^2 = P/EI$

Randbetingelsene gir nå

$$u(0) = 0 \Rightarrow C_2 + \delta + e = 0 \Rightarrow C_2 = -(\delta + e)$$

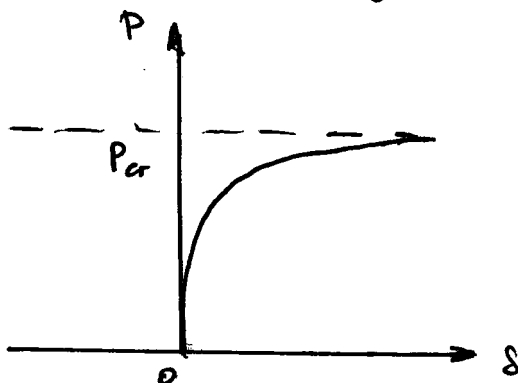
$$u'(0) = 0 \Rightarrow C_1 k - C_2 k \cdot 0 = 0 \Rightarrow C_1 = 0 \text{ som før.}$$

$$u(L) = \delta \Rightarrow C_1 \sin(kL) + C_2 \cos(kL) + \delta + e = \delta$$

$$C_2 \cos(kL) = -e$$

$$\text{dvs } (\delta + e) \cos(kL) = e$$

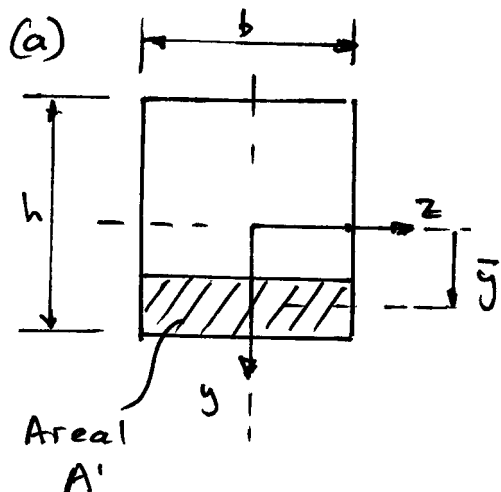
$$\delta = e [\sec(kL) - 1]$$



$$\delta \rightarrow \infty \text{ når } P \rightarrow P_{cr} \text{ (} kL \rightarrow \pi/2 \text{)}$$

Detaljert løsning kreves ikke, men kandidaten må vise at han/hun forstår hva som skjer.

## Oppgave 4



$$I = \frac{1}{12} b h^3$$

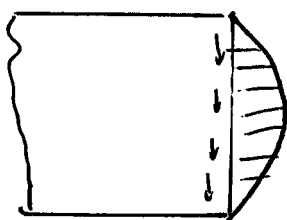
For å beregne  $\tau$  ved avstand  $y$  fra belkeaksen trenger vi  $A'$  og  $\bar{y}$ .

$$A' = b \left( \frac{h}{2} - y \right)$$

$$\bar{y} = \frac{1}{2} \left( y + \frac{h}{2} \right)$$

Fra formelarket finner vi:

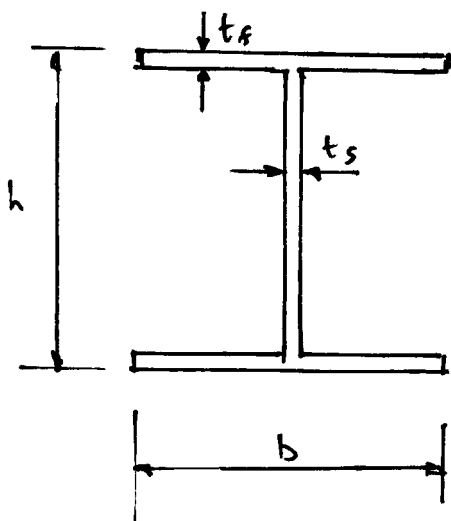
$$\begin{aligned} \tau &= Q \frac{A' \bar{y}}{I b} = Q \frac{b \left( \frac{h}{2} - y \right) \cdot \frac{1}{2} \left( y + \frac{h}{2} \right)}{\frac{1}{12} b h^3 \cdot b} \\ &= \frac{6Q \left[ \left( \frac{h}{2} \right)^2 - y^2 \right]}{b h^3} = \frac{6Q}{b h} \left[ \frac{1}{4} - \left( \frac{y}{h} \right)^2 \right] \end{aligned}$$



Maks verdi av  $\tau$  oppstår ved  $y=0$

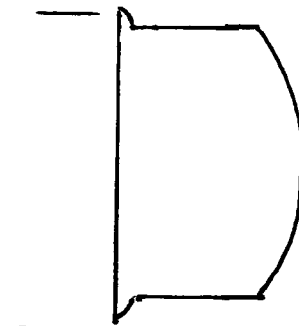
$$\tau_{\text{maks}} = \frac{3Q}{2bh}$$

(b) For en I-bjelke har vi følgende:



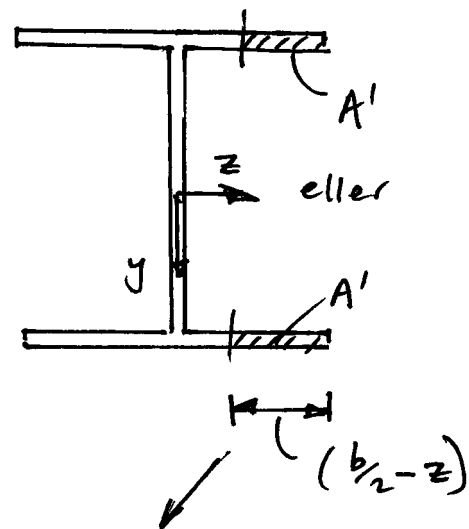
Vertikal skjærspenning  $\tau_{xy}$  ser nå slik ut:

$\tau_{xy}$  i flensene er nå liten sammenlignet med den i steget, fordi  $t_s \ll b$





I flensene er det  $\tau_{xz}$  som er størst. Verdien av  $\tau_{xz}$  i flensene kan finnes ved å se på areal  $A'$  som vist her :



For denne har vi

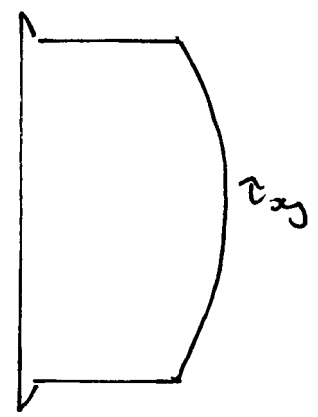
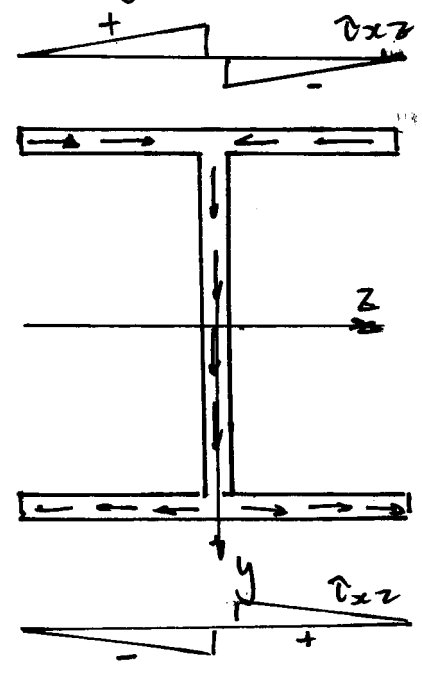
$$A' = t_f \left( \frac{b}{2} - z \right)$$

og  $\bar{y} \approx h/2$

sa  $\tau_{xz} \approx \frac{Q t_f \left( \frac{b}{2} - z \right) h/2}{I_b}$

- lineær variasjon med z

De viktigste skjærspenningene ser slik ut:



(Not to scale!)